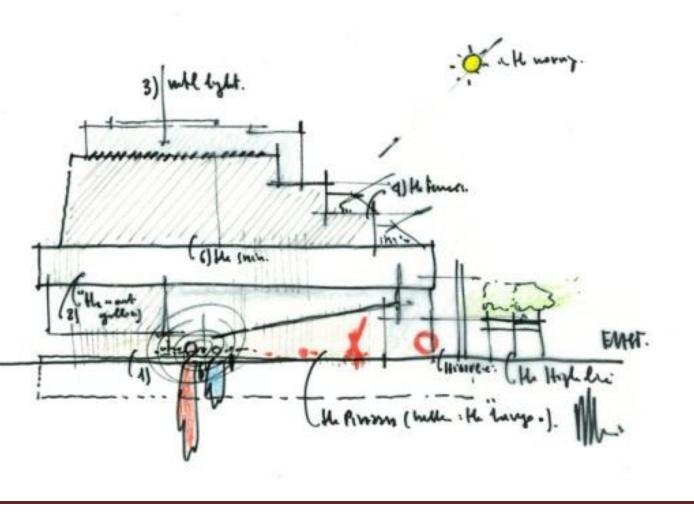
# AMERICAN ART MUSEUM Northeast, United States



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West.

Sean Felton | Structural Advisor: Sustersic

AE Senior Thesis 2013

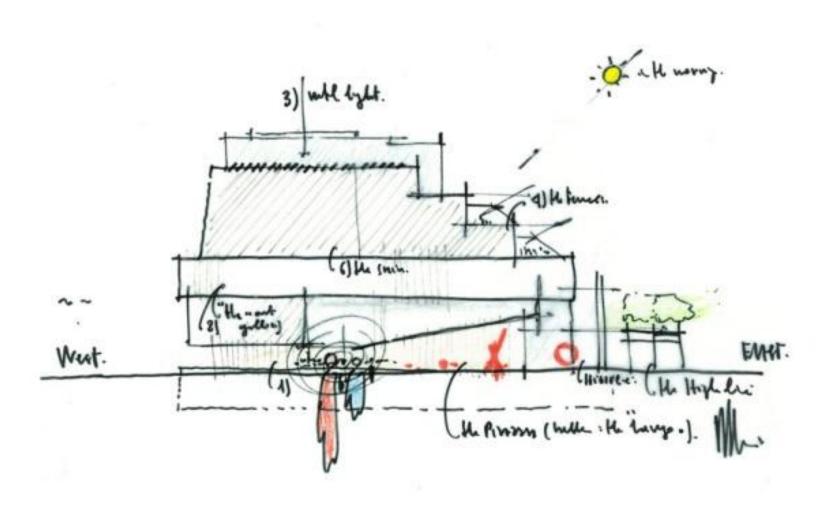


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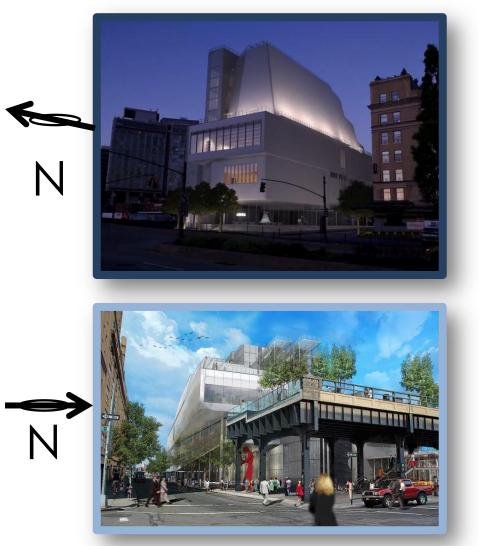
# AMERICAN ART MUSEUM Northeast, United States

# Final Presentation Outline

 Building Introduction Problem Background Proposed Structural System Architecture Considerations Comparative Summary



- Building Introduction
  - **Building Overview**
  - Project Team
- Problem Background
- Proposed Structural System
- Architecture Considerations
- Comparative Summary



- Fully-Functional Facility
- 220,000 sq. ft.  $\bullet$
- 150' tall •
- 9 Stories, Varying Floor Heights
- May 2011 December 2014 •
- Design-Bid-Build; Single Prime Contract \$266,345,323 GMP

# Overview

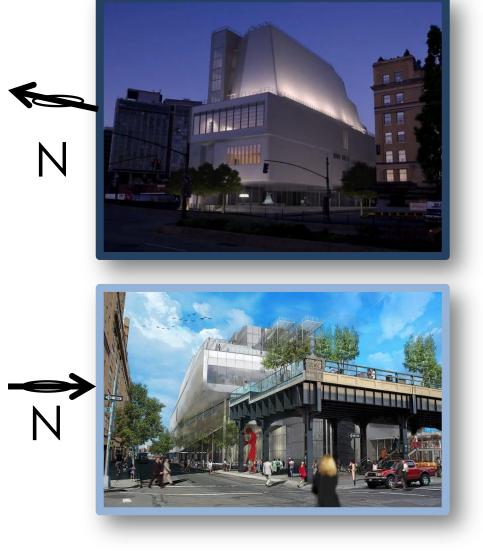


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### AMERICAN ART MUSEUM

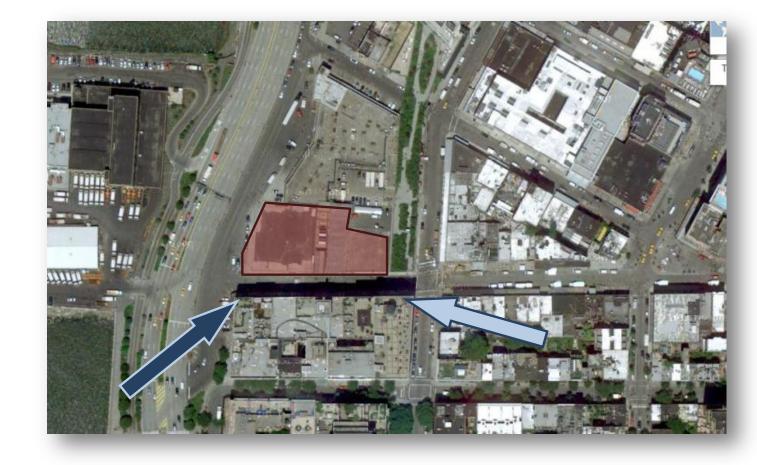
#### www.maps.google.com

- Building Introduction
  - Building Overview
  - Project Team
- Problem Background
- Proposed Structural System
- Architecture Considerations
- Comparative Summary



## Project Team

**Owner:** Not Disclosed General Contractor: Turner Construction **Design Architect:** Renzo Piano Building Workshop **Executive Architect:** Cooper, Robertson & Partners Structural Engineer: Robert Silman Associates Geotechnical: URS Corporation

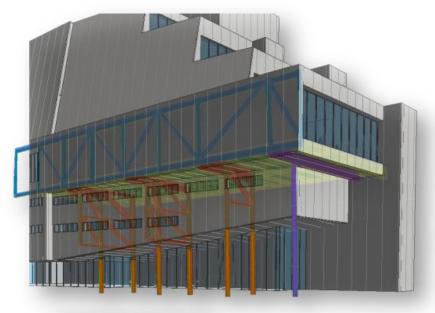


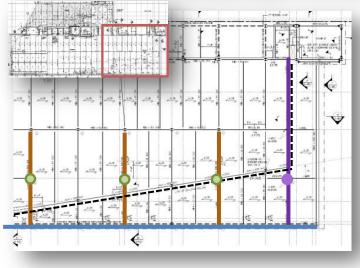
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#### AMERICAN ART MUSEUM

#### www.maps.google.com

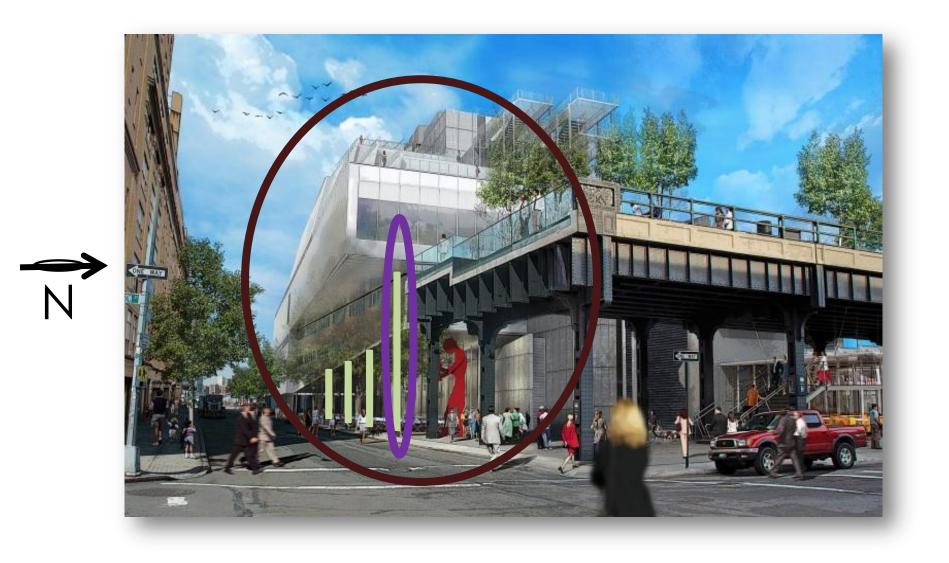
- Building Introduction
- Problem Background
  - Existing System
  - SE Corner Cantilever
  - Problem Statement
- Proposed Structural System
- Architecture Considerations
- Comparative Summary





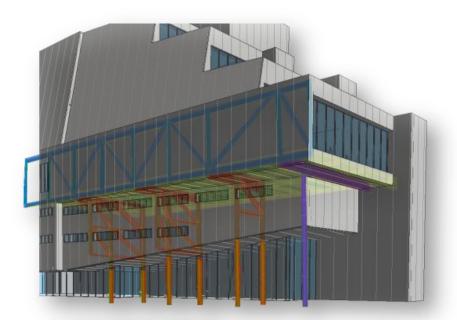
# Problem Statement

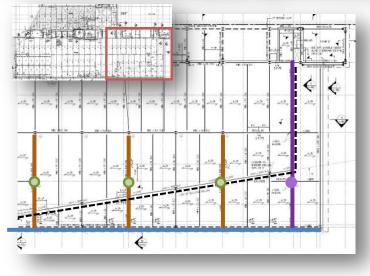
4 Columns Exposed
Column 3-M.5 is Last Support
Exists outside Building Envelope
Architect Request for Removal



#### AMERICAN ART MUSEUM

- Building Introduction
- Problem Background
- Proposed Structural System
  - **Solution Goals** •
  - Load Path Comparison
  - Design Assumptions
  - Truss X •
  - Foundations
  - Deflections •
- Architecture Considerations
- Comparative Summary



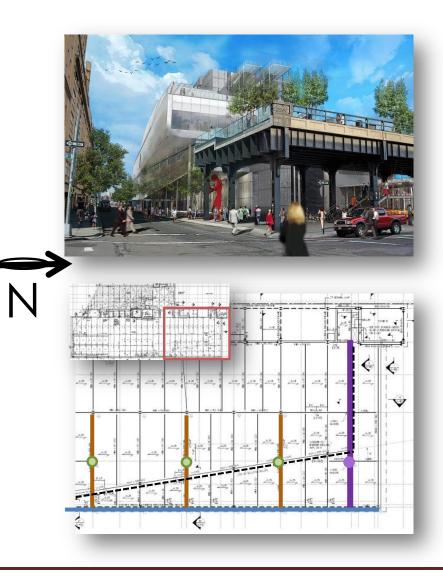


- Structural Stability without 3-M.5 • Serviceability
- Minimize Architectural Impact •
- Minimize Weight and Cost
- Work within Precedence
- Provide Enough Evidence for Decision

# Solution Goals

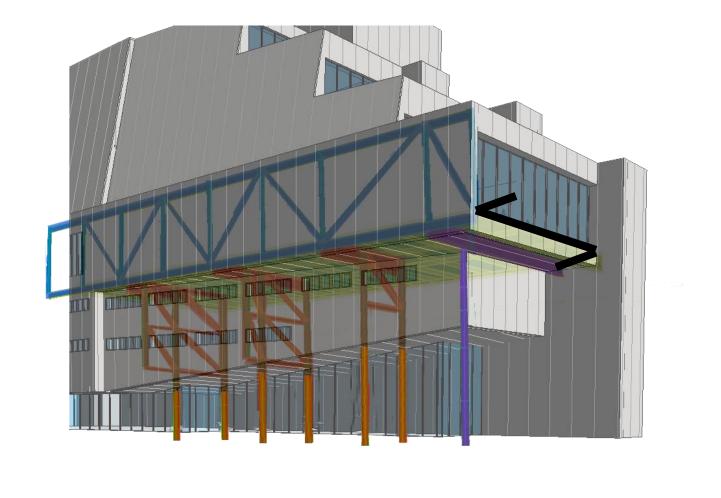


- Building Introduction
- Problem Background
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# CURRENT LOAD PATH

- Floor N-S
- Truss 0.9
- Trusses H, J, L
- 3-M.5, PG46-2

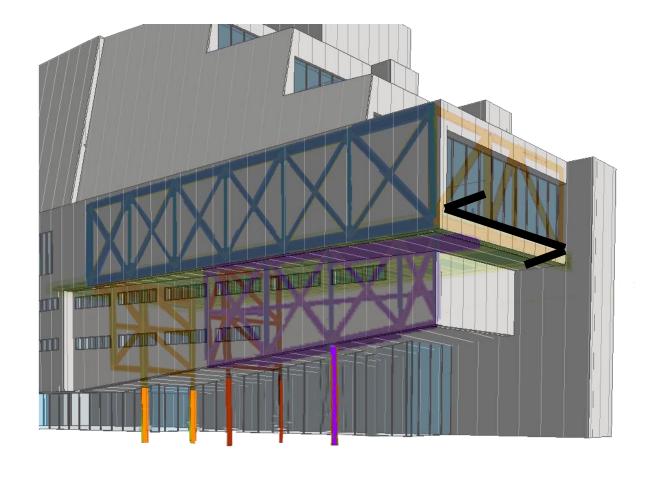


# Proposed Load Path



- Building Introduction
- Problem Background
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  - Solution Goals
  - Load Path Comparison
  - Design Assumptions
  - Truss X
  - Foundations
  - Deflections
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- Comparative Summary

- Floor N-S
- Truss 0.9
- Trusses H, N.2
- Truss X
- Truss J



- Building Introduction
- Problem Background
- Proposed Structural System
  - Solution Goals •
  - Load Path Comparison
  - **Design Assumptions**
  - Truss X
  - Foundations
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### Design Assumptions

# MODELING ASSUMPTIONS

#### • 1.2D + 1.6L + 0.5S

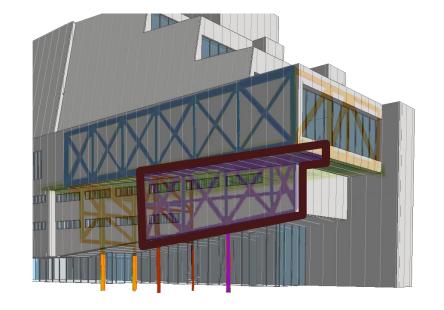
 No Composite Action Concentric Connections No Impact on Lateral System

- Individual Models
- Itemized Reactions
- P-D Effects not Considered

#### AMERICAN ART MUSEUM

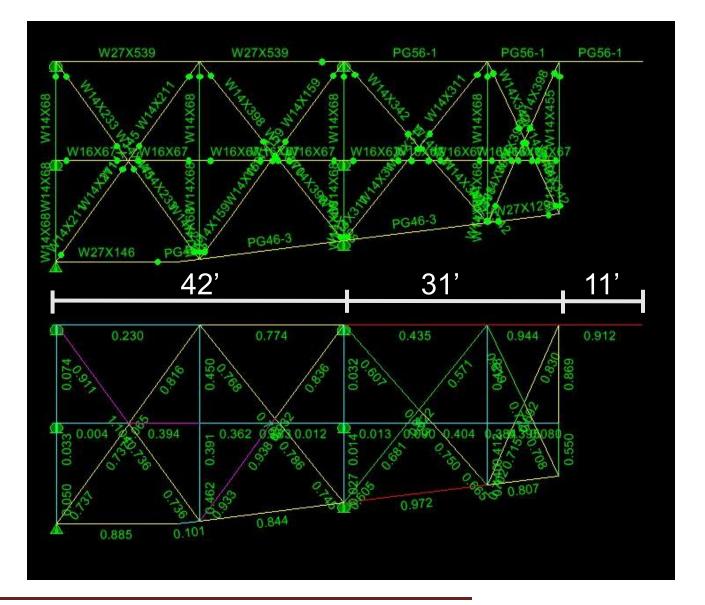
Deflections Checked Separate

- **Building Introduction**
- Problem Background
- Proposed Structural System
  - Solution Goals •
  - Load Path Comparison
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  - Truss X
    - Overview •
    - Custom Members •
    - Summary •
  - Foundations
  - Deflections •
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- Comparative Summary



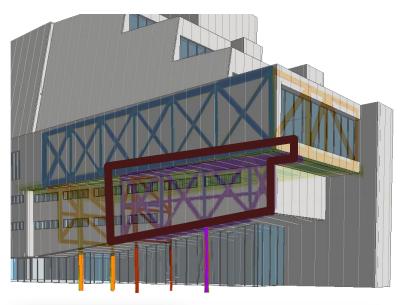
TRUSS X

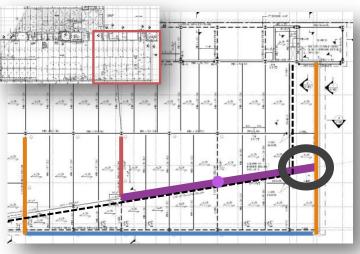
• Point Loads at Panel Points Level 4 Beams for Bracing • Truss J resists uplift



#### AMERICAN ART MUSEUM

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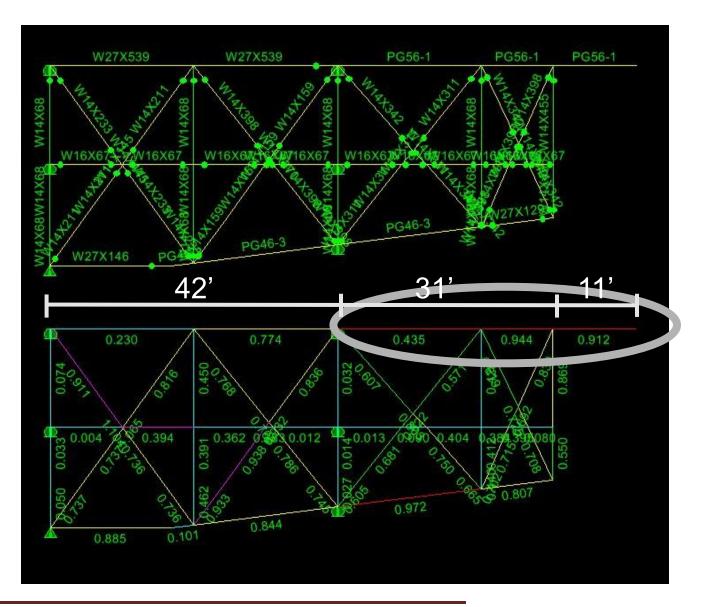




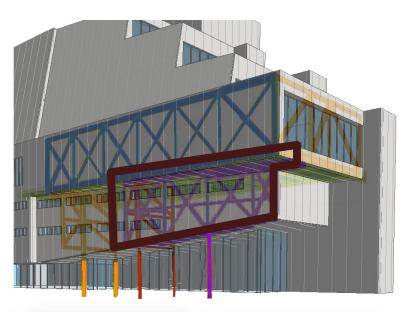
			Axial Capacity					
Member	φMnx (ft-k)	Lp (in)	Lp (ft)	Limit State	KL/r	KL/r lim	φPn (k)	Limit State
32.5	12197	473	39	Yielding	19.4	113	8395	Torsion
33-1	12518	479	40	Yielding	19.2	113	9446	Torsion
44-1	20520	609	51	Yielding	14.7	113	9532	Torsion
46-1	12555	648	54	Yielding	SL	SL	SL	SL
46-2	29550	657	55	Yielding	13.7	113	16775	Torsion
46-3	22170	631	53	Yielding	14.1	113	9724	Torsion
72-1	45090	815	68	Yielding	10.7	113	10174	Torsion

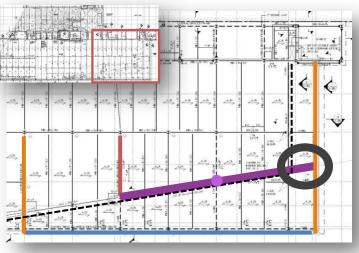
- $M_U = 40,700 \text{ ft-k}$
- Vu = 3200 k
- $T_U = 1580 \text{ k}$

Level 5 D = 1340 k L = 945 k S = 2 k $P_U = 3200 \text{ k}$ 



- Building Introduction
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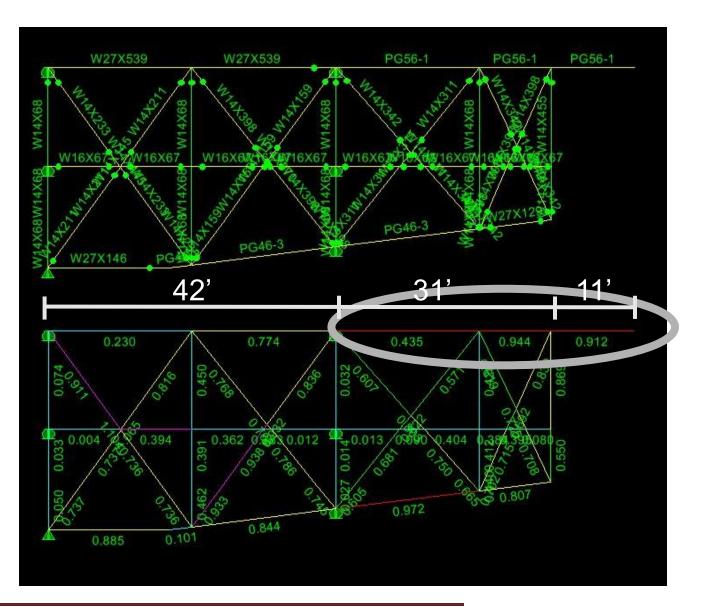




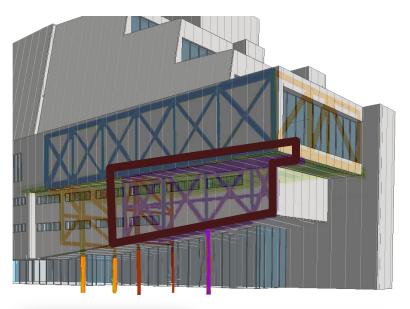
			Axial Capacity					
Member	φMnx (ft-k)	Lp (in)	Lp (ft)	Limit State	KL/r	KL/r lim	φPn (k)	Limit State
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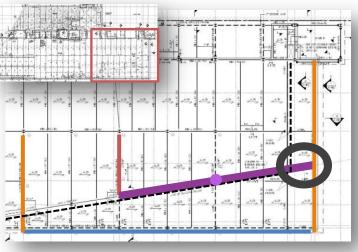
- $M_U = 40,700 \text{ ft-k}$
- Vu = 3200 k
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Level 5 D = 1340 k L = 945 k S = 2 k $P_U = 3200 \text{ k}$ 



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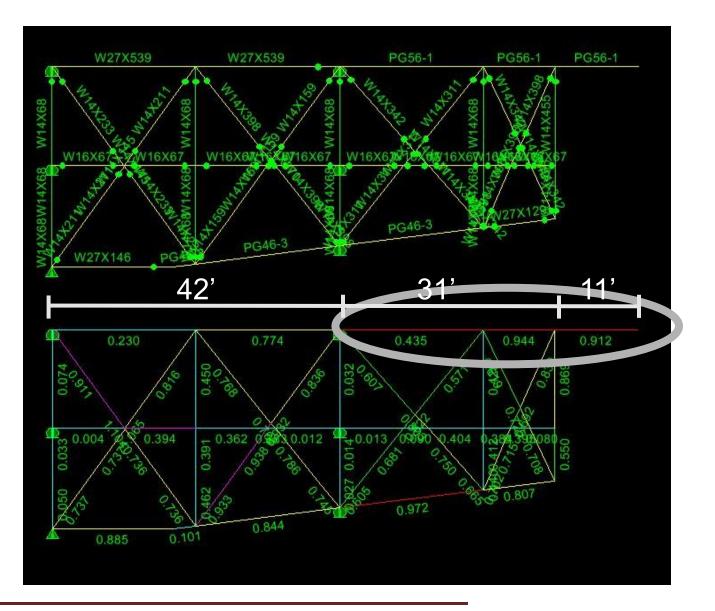




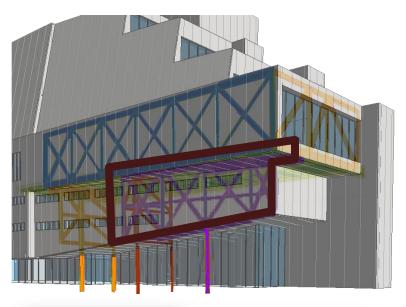
- Mu = 40,700 ft-k
- Vu = 3200 k
- Tu = 1580 k
- Achieves 94% Efficiency

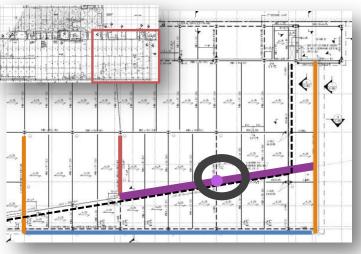
PG56-1			Capacity	
20	ft	φMn	41571	ft-k
56	in	φVn	3402	k
24	in	φTn	25245	k
10	in	φPn	27541	k
2.25	in			

Level 5 D = 1340 k L = 945 k S = 2 k $P \cup = 3200 \text{ k}$ 



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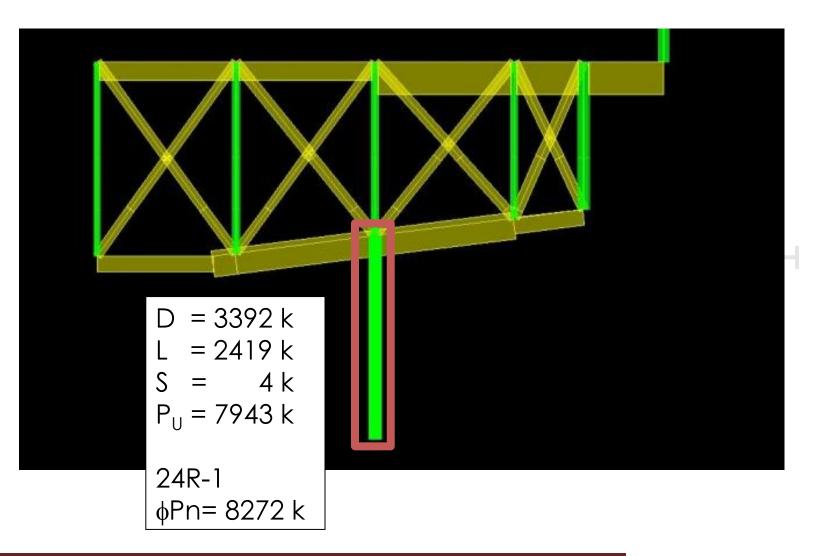




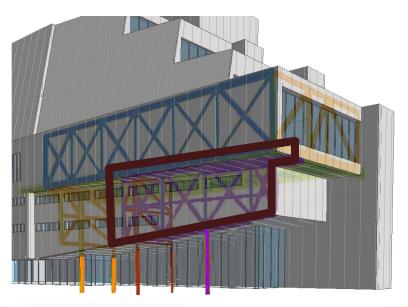


TRUSS X

ape	Lu	φMn	φPn	φTn
5A	25	750	2421	2295
5B	25	624	2161	1685
22	25	1714	4389	3545

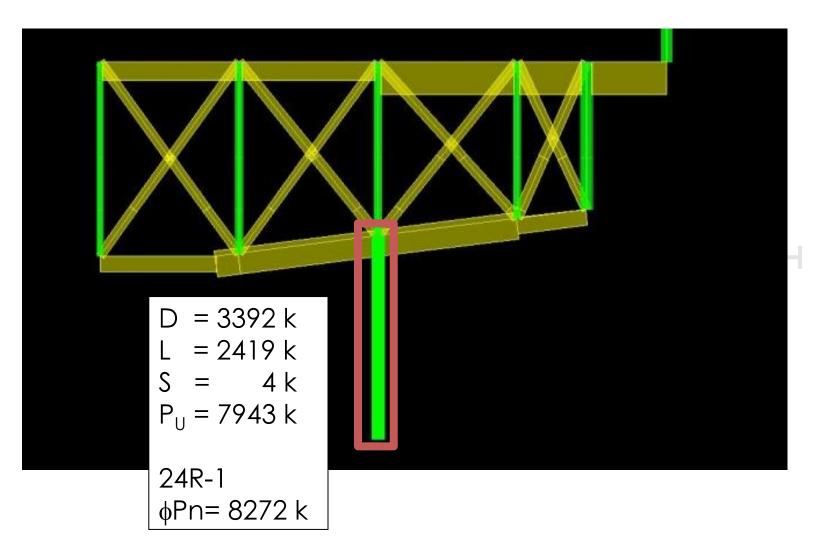


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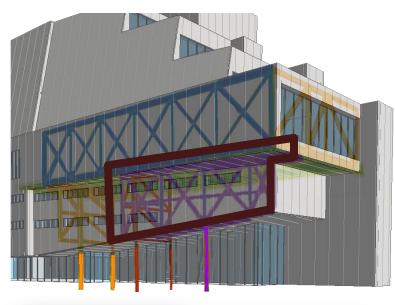




Pipe					
Do	24	in			
t	1.75	in			
Concrete					
f'c	15000	psi			
fy	150	ksi			
no.	11				
n	16				
	Capacity				
φPn	8272	k			
φTn	8053	k			
φMn	2754	ft-k			



- Building Introduction
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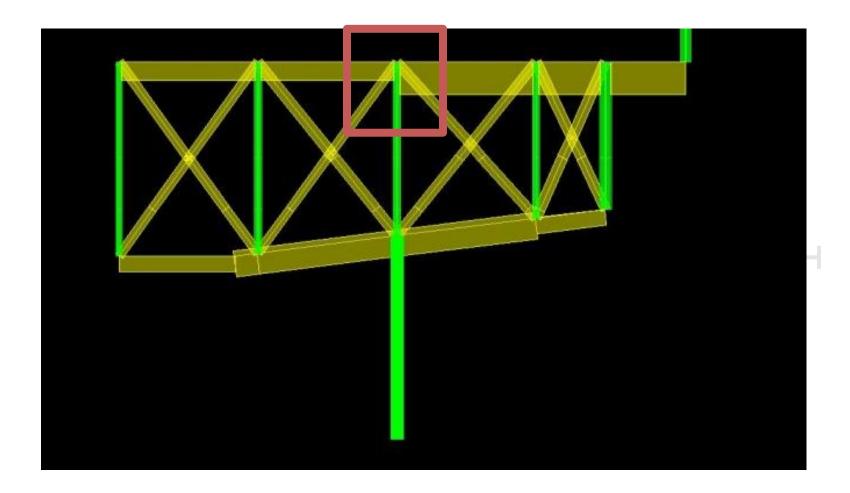


## Truss X

- Multiple Custom Sections
  - PG56-1
  - PG46-3
  - 24R-1

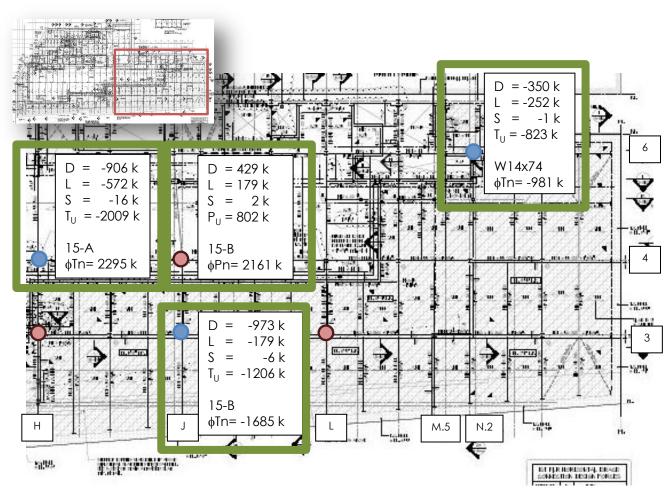
 $\bullet$ 

- W14x 68 W14x455 W27x539 Eccentricity Issues
- Final Weight: 121.6 t



- Building Introduction
- Problem Background
- Proposed Structural System
  - Solution Goals
  - Load Path Comparison
  - Design Assumptions
  - Truss X
  - Foundations
    - Truss Supports
    - Caissons
  - Deflections
- Architecture Considerations
- Comparative Summary



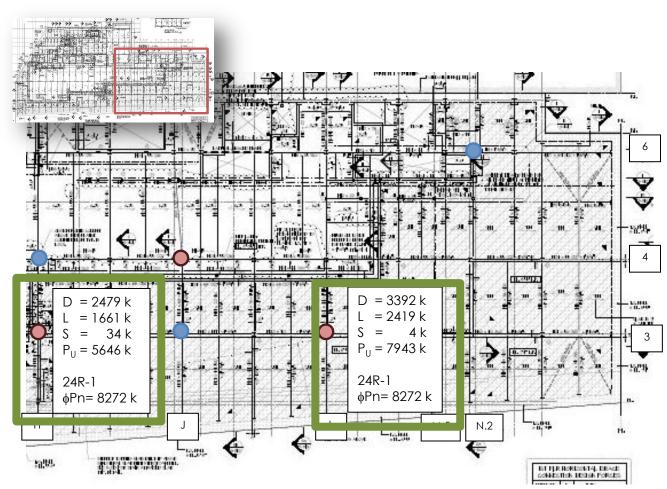


# Truss Supports

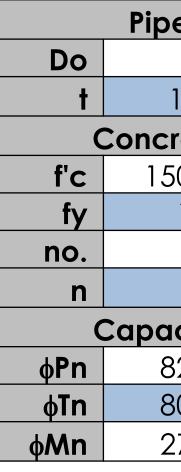
Shape	Lu	φMn	φPn	φTn
15A	25	750	2421	2295
15B	25	624	2161	1685
22	25	1714	4389	3545

- Building Introduction
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    - Truss Supports
    - Caissons
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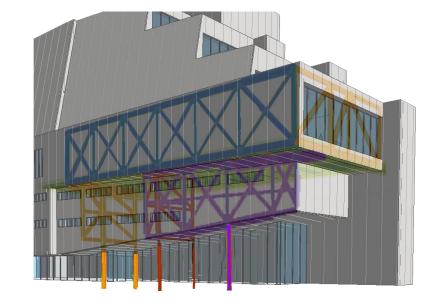


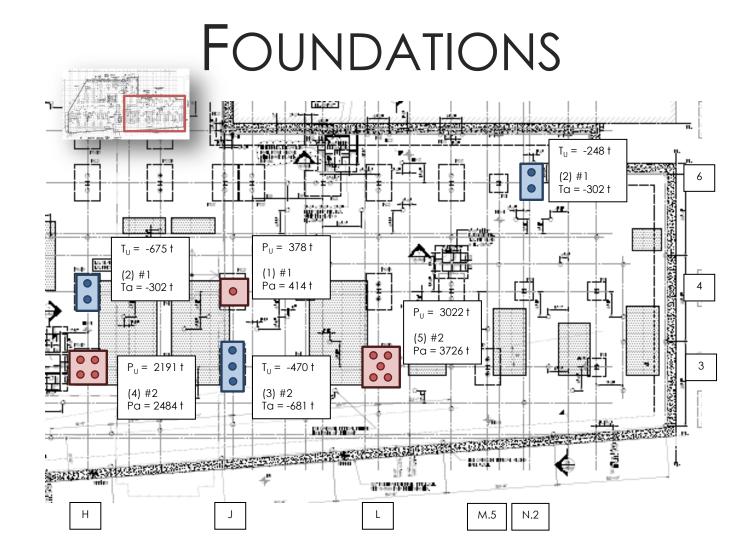
# Truss Supports



е	
24	in
1.75	in
rete	
5000	psi
150	ksi
11	
16	
city	
3272	k
8053	k
2754	ft-k

- **Building Introduction**
- Problem Background
- Proposed Structural System
  - Solution Goals •
  - Load Path Comparison
  - Design Assumptions
  - Tr∪ss X •
  - Foundations •
    - Truss Supports
    - Caissons •
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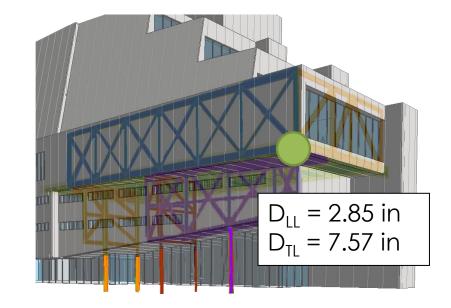


	CAISSON SCHEDULE							
	MARK	NOTES	CAISSON O.D.	CASING	CAISSON REINF. VERT. BARS	MIN. DEPTH OF ROCK SOCKET*	TENSION CAPACITY (TONS)	COMPRESSION CAPACITY (TONS)
1	[0]	TYPICAL, @PC/WALL	13.375"	½" THICK Fy=80 ksi	#24	'-O"	151	414
2	æ	HIGH CAPACITY	13.375"	½" THICK Fy=80 ksi	2 #24	16'-0"	227	621
3	0	TCI - NOT @PC/WALL	9.875"	½" THICK Fy=80 ksi	#24	15'-0"	151	୩

### American Art Museum

#### D+H+F+L+S+T

- Building Introduction
- Problem Background
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  - Load Path Comparison
  - Design Assumptions
  - Truss X
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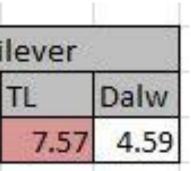
- = 45'-10'' Cantilever • IBC Chapter 16  $11/1 \rightarrow 1/2/0$  Live Load Total Load

## Deflections & Serviceability

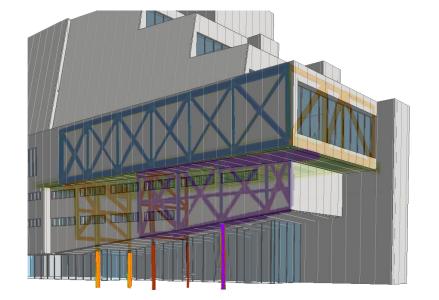
$$= (\frac{1}{2}) \times \frac{1}{360} = \frac{1}{180}$$
$$= (\frac{1}{2}) \times \frac{1}{240} = \frac{1}{120}$$

	Canti
LL	Dalw
2.85	3.06

#### AMERICAN ART MUSEUM



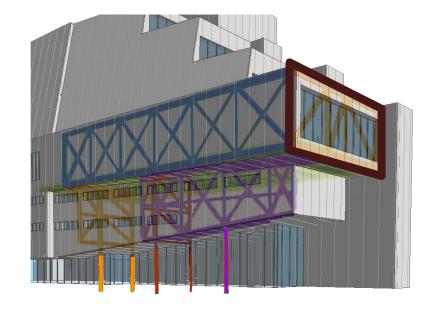
# ARCHITECTURE CONSIDERATIONS



- Building Introduction
- Problem Background
- Proposed Structural System
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- Comparative Summary

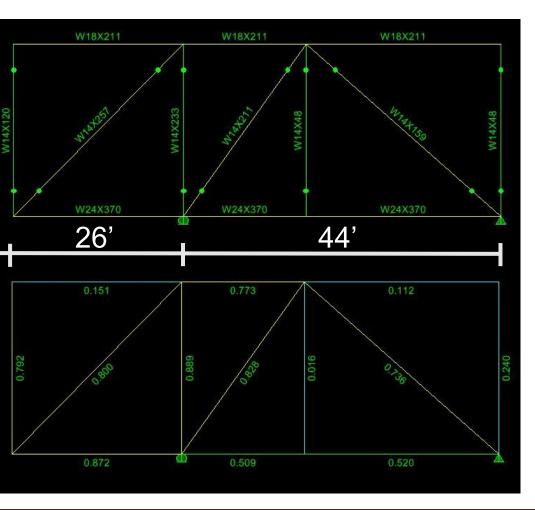
 Minimize Impact of Architecture Open Office Spaces Panel Module Alignment • Architectural Envelope Façade and Glazing

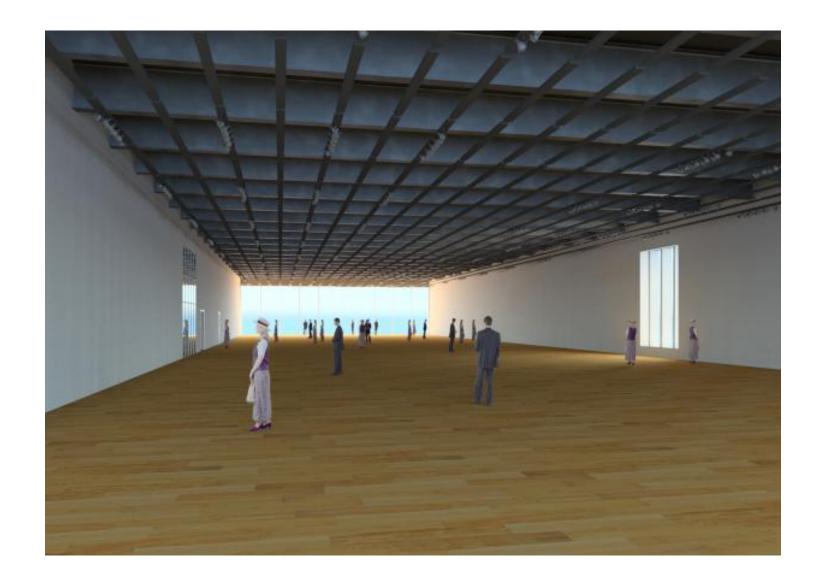
AMERICAN ART MUSEUM



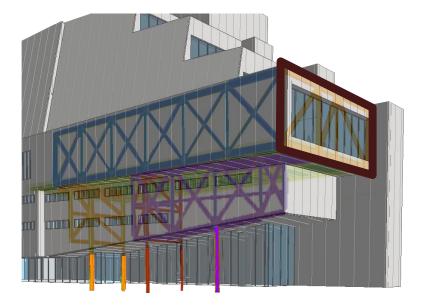
- **Building Introduction**
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## PANEL MODULE ALIGNMENT



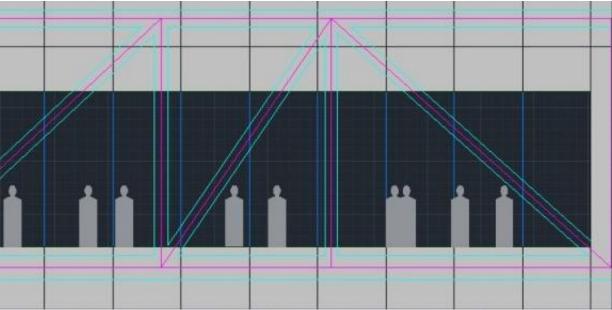


# Panel Module Alignment

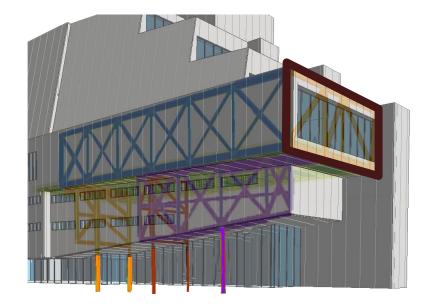


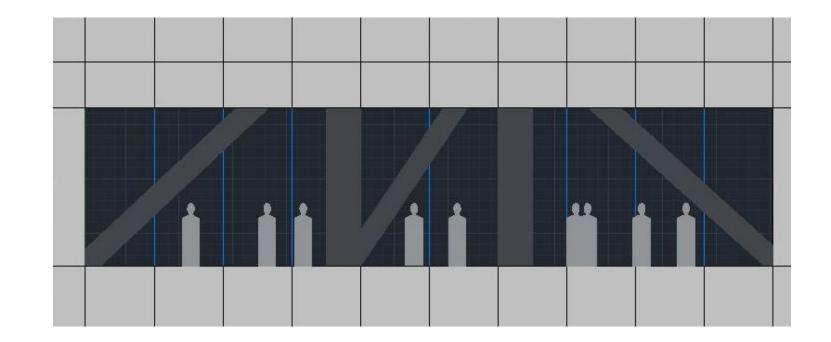


- Building Introduction
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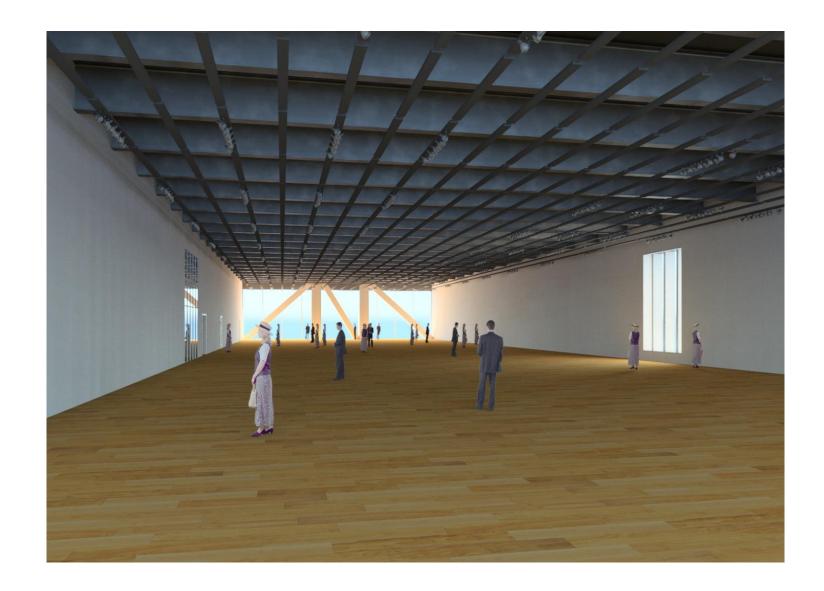




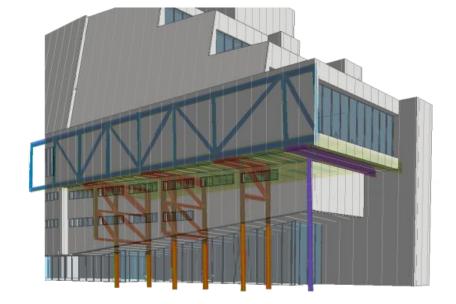


- **Building Introduction**
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## PANEL MODULE ALIGNMENT

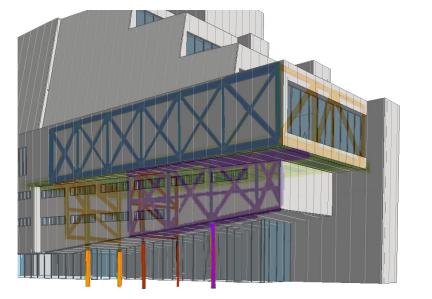






- **Building Introduction**
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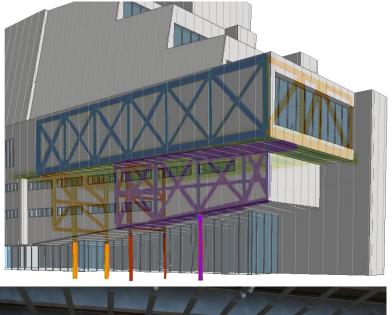
## COMPARATIVE SUMMARY



Structural Concerns	Cu
Remove Column 3-M.5	
No. of Steps in Load Path	
Max. Element Weight	
Overall Weight	
Max. Pile Group	
No. of Custom Sections	
Columns Max.O.D.	
Max. f'c	
Max. Total Deflection	
Acceptable Deflections	
Architectural Concerns	Cu
Gallery Interference	
Wall X Interference	
Remove Truss L	
Maintain Web Openings	
Maintain Building Envelope	
Construction Concerns	Cu
No. of Long Trusses	
Cost of Superstructure	
Cost of Foundations	
Total Structural Cost	
Total Difference	
Sector sect	

rent Design	Proposed Design
NO	YES
2	4
45.1 t	121.6 t
199.5 t	297.1 t
2	5
10	12
22"	24"
5,000	15,000
-	-7.57 in
YES	NO
rent Design	Proposed Design
NO	Truss N.2
NO	Truss X
NO	YES
YES	YES
YES	YES
rent Design	Proposed Design
1	2
\$3,928,000	\$5,946,000
\$238,000	335,000
\$4,166,000	\$6,281,000
	\$2,115,000

- Building Introduction
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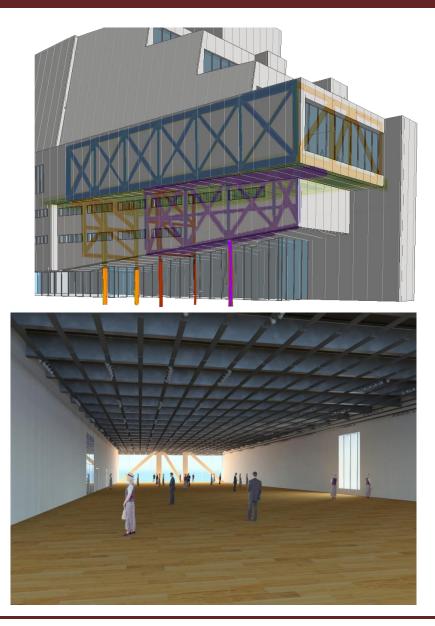
### ACKNOWLEDGEMENTS

- The Owner
- Turner Construction
- The AE Faculty
- Classmates
- Family, Friends

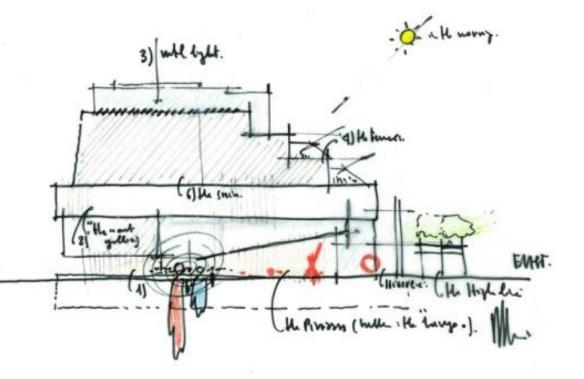
	<u> </u>
Structural Concerns	Cur
Remove Column 3-M.5	
No. of Steps in Load Path	
Max. Element Weight	
Overall Weight	
Max. Pile Group	
No. of Custom Sections	
Columns Max.O.D.	
Max. f'c	
Max. Total Deflection	
Acceptable Deflections	
Architectural Concerns	Cur
Gallery Interference	
Wall X Interference	
Remove Truss L	
Maintain Web Openings	
Maintain Building Envelope	
Construction Concerns	Cur
No. of Long Trusses	
Cost of Superstructure	
Cost of Foundations	
Total Structural Cost	
Total Difference	
	-

rent Design	Proposed Design	
NO	YES	
2	4	
45.1 t	121.6 t	
199.5 t	297.1 t	
2	5	
10	12	
22"	24"	
5,000	15,000	
-	-7.57 in	
YES	NO	
rent Design	Proposed Design	
NO	Truss N.2	
NO	Truss X	
NO	YES	
YES	YES	
YES	YES	
rent Design	Proposed Design	
1	2	
\$3,928,000	\$5,946,000	
\$238,000	335,000	
\$4,166,000	\$6,281,000	
	\$2,115,000	

- Building Introduction
- Problem Background
- Proposed Structural System
- Architecture Considerations
- Comparative Summary



# JURY QUESTIONS?



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West.



| Structural Concerns        | Current Design  | Proposed Design  |
|----------------------------|-----------------|------------------|
| Remove Column 3-M.5        | NO              | YES              |
| No. of Steps in Load Path  | 2               | 4                |
| Max. Element Weight        | 45.1 t          | 121.6 t          |
| Overall Weight             | 199.5 t         | 297.1 t          |
| Max. Pile Group            | 2               | 5                |
| No. of Custom Sections     | 10              | 12               |
| Columns Max.O.D.           | 22"             | 24"              |
| Max. f'c                   | 5,000           | 15,000           |
| Max. Total Deflection      | -               | -7.57 in         |
| Acceptable Deflections     | YES             | NO               |
| Architectural Concerns     | ICurrent Design | IProposed Design |
| Gallery Interference       | NO              | Truss N.2        |
| Wall X Interference        | NO              | Truss X          |
| Remove Truss L             | NO              | YES              |
| Maintain Web Openings      | YES             | YES              |
| Maintain Building Envelope | YES             | YES              |
| Construction Concerns      | Current Design  | Proposed Design  |
| No. of Long Trusses        | 1               | 2                |
| Cost of Superstructure     | \$3,928,000     | \$5,946,000      |
| Cost of Foundations        | \$238,000       | 335,000          |
| Total Structural Cost      | \$4,166,000     | \$6,281,000      |
| Total Difference           |                 | \$2,115,000      |